

ME 305 Fluid Mechanics I

Part 8

Viscous Flow in Pipes and Ducts

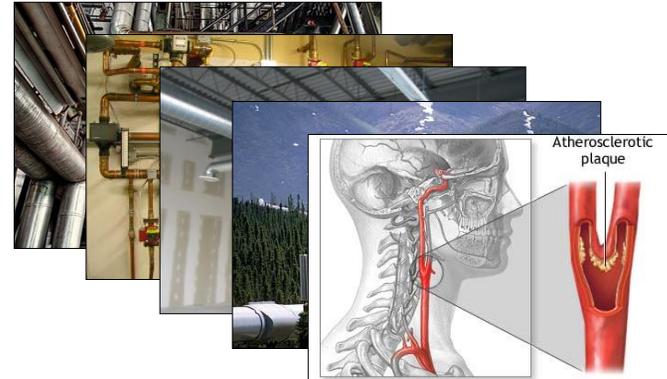
These presentations are prepared by
Dr. Cüneyt Sert
Department of Mechanical Engineering
Middle East Technical University
Ankara, Turkey
csert@metu.edu.tr

You can get the most recent version of this document from Dr. Sert's web site.
Please ask for permission before using them to teach. You are NOT allowed to modify them.

8-1

Flow in Pipes and Ducts

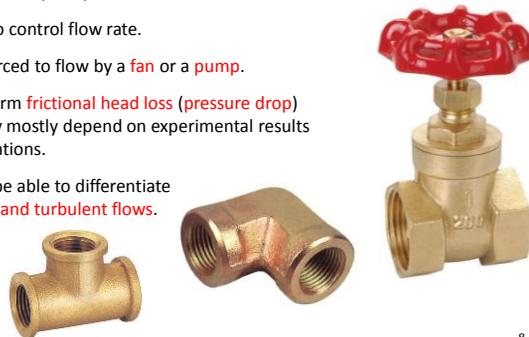
- Flow in closed conduits (circular **pipes** and non-circular **ducts**) are very common.



8-2

Flow in Pipes and Ducts (cont'd)

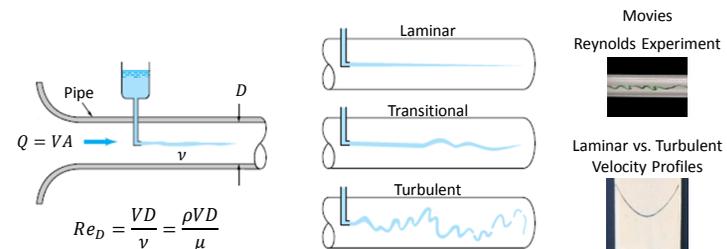
- We assume that pipes/ducts are completely filled with fluid. Other case is known as open channel flow.
- Typical systems involve pipes/ducts of various sizes connected to each other by various **fittings, elbows, tees**, etc.
- Valves** are used to control flow rate.
- Fluid is usually forced to flow by a **fan** or a **pump**.
- We need to perform **frictional head loss (pressure drop)** calculations. They mostly depend on experimental results and empirical relations.
- First we need to be able to differentiate between **laminar and turbulent flows**.



8-3

Laminar vs. Turbulent Flows

- Laminar flow** is characterized by smooth streamlines and highly ordered motion. Fluid flows as if there are immiscible layers of fluid.
- Turbulent flow** is highly disordered. Usually there are unsteady, random fluctuations on top of a mean flow.
- Most flows encountered in practice are turbulent.



8-4

Laminar vs. Turbulent Flows (cont'd)

- For pipe flow, **transition** from laminar to turbulent flow occurs at a critical Reynolds number of approximately 2300.

$$Re_D < 2300 : \text{Laminar}$$

$$Re_D > 2300 : \text{Turbulent}$$

$$Re = \frac{\text{Inertia forces}}{\text{Viscous forces}} = \frac{VD}{\nu}$$

Characteristic speed
 Characteristic length

- At large Reynolds numbers **inertia forces**, which are proportional to $\rho V^2 D^2$, are much higher than the **viscous forces**. Viscous forces can not regularize random fluctuations. Fluctuations grow and the flow becomes turbulent.
- At small Reynolds numbers **viscous forces** are high enough to suppress random fluctuations and keep the flow in order, i.e. laminar.

Exercise: Estimate the typical Reynolds number of the flow inside the pipe that supplies water to your shower.

8-5

Pressure Loss (Pressure Drop, Head Loss) in Pipe Flow

- As a fluid flows in a straight, constant diameter pipe its pressure drops due to viscous effects, known as **major pressure loss**.
- Additional pressure drops occur due to other components such as valves, bends, tees, sudden expansions, sudden contractions, etc. These are known as **minor pressure losses**.
- In Chapter 6 we studied analytical solution of **Hagen-Poiseuille flow**, which is the steady, fully-developed, laminar flow in a circular pipe.
- We showed that $\frac{dp}{dx} = \text{constant}$ and pressure drop over a pipe section of length L is

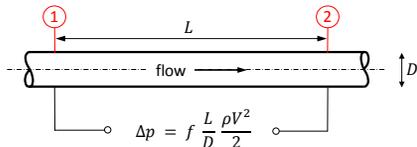
$$\Delta p = f \frac{L}{D} \frac{\rho V^2}{2}$$

Darcy friction factor Average speed Dynamic pressure

where $f = 64/Re_D$ for laminar flow, with $Re_D = \frac{VD}{\nu}$.

8-6

Major Pressure Loss in Laminar Pipe Flow



- Consider the EBE between sections 1 and 2.

$$\frac{p_1}{\rho g} + \frac{V_1^2}{2g} + z_1 = \frac{p_2}{\rho g} + \frac{V_2^2}{2g} + z_2 + h_f$$

- Friction head is related to the pressure drop as

$$h_f = \frac{\Delta p}{\rho g} = f \frac{L}{D} \frac{V^2}{2g}$$

- The above important equation is known as the **Darcy-Weisbach equation**.

8-7

Major Pressure Loss in Turbulent Pipe Flow

- Analytical solution of pipe flow is available only for laminar flows.
- It is not possible to solve Navier-Stokes equations analytically for turbulent flows.
- Instead we need to determine pressure drop values experimentally.
- For a turbulent pipe flow pressure drop depends on

$$\Delta p = \Delta p(L, D, V, \mu, \rho, \epsilon)$$

New parameter:
Surface roughness

- A Buckingham-Pi analysis yields the following relation

$$\frac{\Delta p}{\rho V^2 / 2} = F\left(\frac{L}{D}, Re_D, \frac{\epsilon}{D}\right)$$

which can be expressed in the same way as laminar flow $\Delta p = f \left(Re_D, \frac{\epsilon}{D}\right) \frac{L}{D} \frac{\rho V^2}{2}$

8-8

Major Pressure Loss in Turbulent Pipe Flow (cont'd)

- For turbulent pipe flows friction factor is a function of Reynolds number and relative surface roughness.

$$f \left(Re_D, \frac{\epsilon}{D} \right)$$

Relative surface roughness

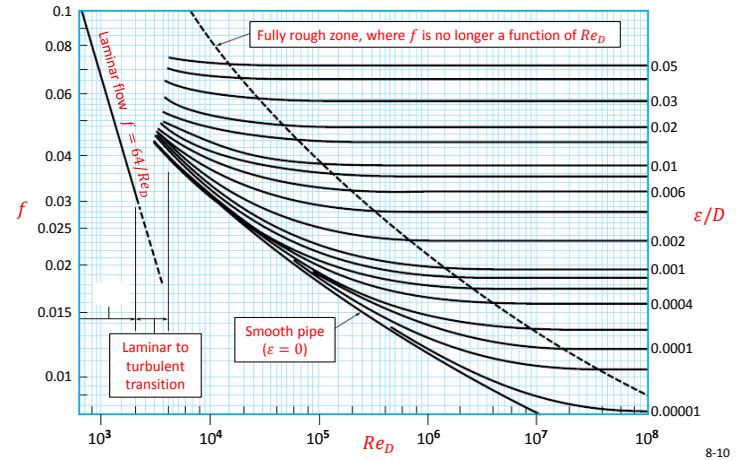
- In 1933 **Nikuradse** performed very detailed experiments to determine friction factor for turbulent flows.
- Later **Colebrook** presented this experimental data as the following equation

$$\text{Colebrook Formula : } \frac{1}{\sqrt{f}} = -2 \log \left(\frac{\epsilon/D}{3.7} + \frac{2.51}{Re_D \sqrt{f}} \right)$$

- An easy to use graphical representation of this equation is known as the **Moody diagram**, given in the next slide).
- Be careful in reading the logarithmic horizontal and vertical axes of the Moody diagram.

8-9

Moody Diagram to Read Friction Factor



8-10

Moody Diagram (cont'd)

- To use the Moody diagram first we read the roughness of the pipe of interest from a reference (see the next slide).
- Next we calculate the relative roughness (ϵ/D) and Reynolds number, and read the friction factor value f .
- $f = 64/Re_D$ relation for laminar flows appears in the Moody diagram as a straight line.
- Even for **smooth pipes** ($\epsilon = 0$) friction factor is not zero, as expected.
- For high Re_D values f becomes independent of Re_D . This region is known as **fully rough (or fully turbulent) flow**.
- Between the laminar and turbulent regions, there is a **transition region** ($2100 < Re_D < 4000$), where the experimental data is not very reliable.
- We do not expect to read f with more than 10% accuracy from the Moody diagram.

8-11

Moody Diagram (cont'd)

Pipe	Roughness, ϵ Millimeters
Riveted steel	0.9–9.0
Concrete	0.3–3.0
Wood stave	0.18–0.9
Cast iron	0.26
Galvanized iron	0.15
Commercial steel or wrought iron	0.045
Drawn tubing	0.0015
Plastic, glass	0.0 (smooth)

For pipes which are in service for a long time, roughness values given in tables for new pipes should be used with caution.



- As an alternative to the Moody diagram, **Haaland's equation (explicit and easy to use)** can be used to calculate f .

$$\frac{1}{\sqrt{f}} = -1.8 \log \left(\left(\frac{\epsilon/D}{3.7} \right)^{1.11} + \frac{6.9}{Re_D} \right)$$

8-12

Example

Exercise: Crude oil flows through a level section of a pipeline at a rate of 2.944 m³/s. The pipe inside diameter is 1.22 m, its roughness is equivalent to galvanized iron. The maximum allowable pressure is 8.27 MPa. The minimum pressure required to keep dissolved gases in solution in the crude oil is 344.5 kPa. The crude oil has a specific gravity of 0.93. Viscosity at the pumping temperature of 60 °C is $\mu = 0.0168$ Pa s. a) Determine the maximum allowable spacing between the pumping stations. b) If the pump efficiency is 85%, determine the power that must be supplied at each pumping station. (Reference: Fox's book)



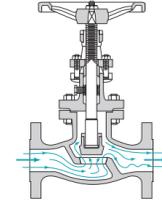
8-13

Minor Head Losses

- Pressure drops due to the flow through valves, bends, tees, sudden area changes, etc. are known as minor losses.
- Minor head losses can be calculated as

$$h_f = k \frac{V^2}{2g}$$

Head loss coefficient



- For certain piping system elements minor losses are given in terms of an **equivalent length**

$$h_f = f \frac{L_e}{D} \frac{V^2}{2g}$$

where L_e is the length of a straight pipe section that would create a major loss equal to the minor loss created by the element. f is obtained from the Moody diagram.

- k and L_e values are obtained from a figure or table, such as the ones given in the coming slides.

8-14

Minor Head Losses (cont'd)

Minor Loss Coefficients for Pipe Entrances

Entrance Type	Minor Loss Coefficient, K^a								
Reentrant	0.78								
Square-edged	0.5								
Rounded	<table border="1" style="display: inline-table;"> <tr> <td>r/D</td> <td>0.02</td> <td>0.06</td> <td>≥ 0.15</td> </tr> <tr> <td>K</td> <td>0.28</td> <td>0.15</td> <td>0.04</td> </tr> </table>	r/D	0.02	0.06	≥ 0.15	K	0.28	0.15	0.04
r/D	0.02	0.06	≥ 0.15						
K	0.28	0.15	0.04						

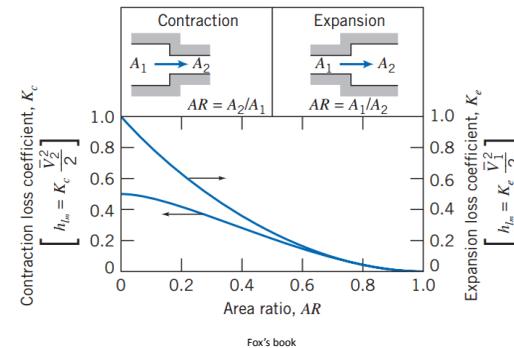
^aBased on $h_{L_e} = K(\bar{V}^2/2)$, where \bar{V} is the mean velocity in the pipe.

Fox's book

8-15

Minor Head Losses (cont'd)

Loss coefficients for flow through sudden area changes



Fox's book

8-16

Examples (cont'd)

- Certain problems require an iterative solution because Re_D and f cannot be computed directly. Solution starts with an initial guess for f and/or D . Usually a couple of iterations are enough to get the final answer.
- Typical problems of this sort are
 - L and D of the pipe are known. Find Q for a specified Δp .
 - L of the pipe is known. Find D for a specified Q and Δp .
 - Multiple path systems

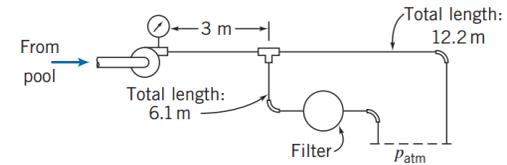
Exercise : Air at 1 atm and 35 °C is to be transported in a 150 m long plastic pipe at a rate of 0.35 m³/s. If the head loss in the pipe is not to exceed 20 m, determine the minimum diameter of the duct.

Exercise : Reconsider the previous example. Now the duct length is doubled while its diameter is maintained constant. If the total head loss is to remain constant, determine the new flow rate through the duct.

8-21

Examples (cont'd)

Exercise : A swimming pool has a partial-flow filtration system. Water at 24 °C is pumped from the pool through the system shown. The pump delivers 1.9 L/s. The pipe is nominal 20 mm PVC (internal diameter 20.93 mm). The pressure loss through the filter is approximately $\Delta p = 1039 Q^2$, where Δp is in kPa and Q is in L/s. Determine the pump pressure and the flow rate through each branch of the system (Reference: Fox's book).



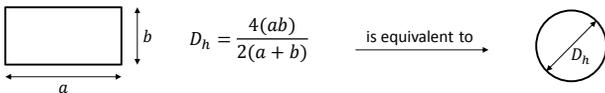
8-22

Non-Circular Geometries

- Correlations derived for circular pipes can be used for noncircular ones, provided that the cross sections are not too exaggerated.
- Ducts with square or rectangular cross sections can be treated if their height to width ratio is less than about 3 or 4.
- To do this we use the **hydraulic diameter** concept

$$D_h = \frac{4A}{P}$$

↙ Cross sectional area
↘ Wetted perimeter (length of wall in contact with fluid at any cross-section)



8-23